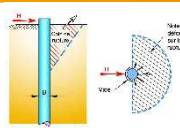
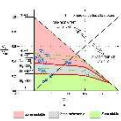
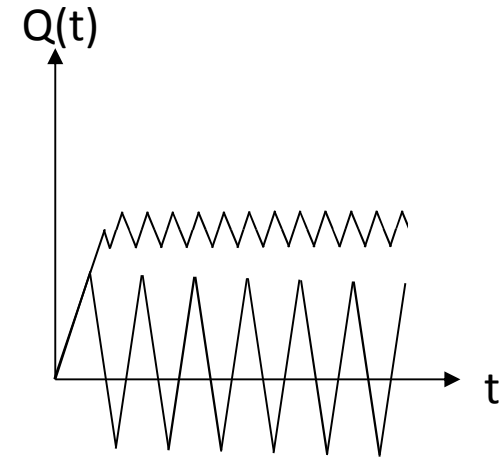
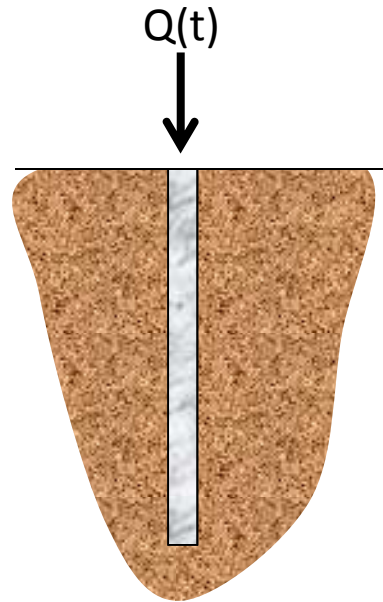


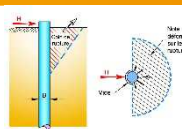
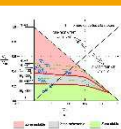
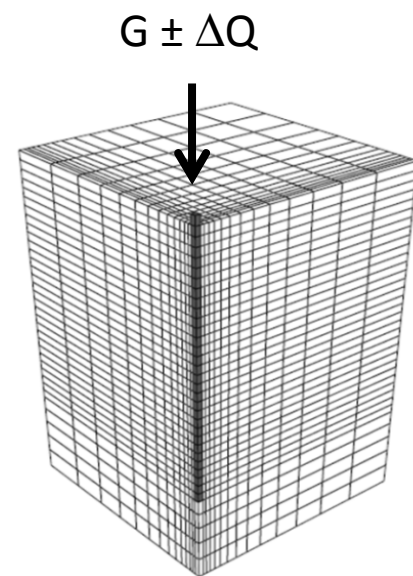
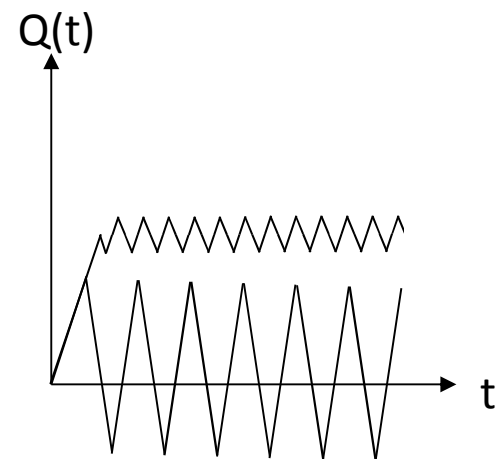
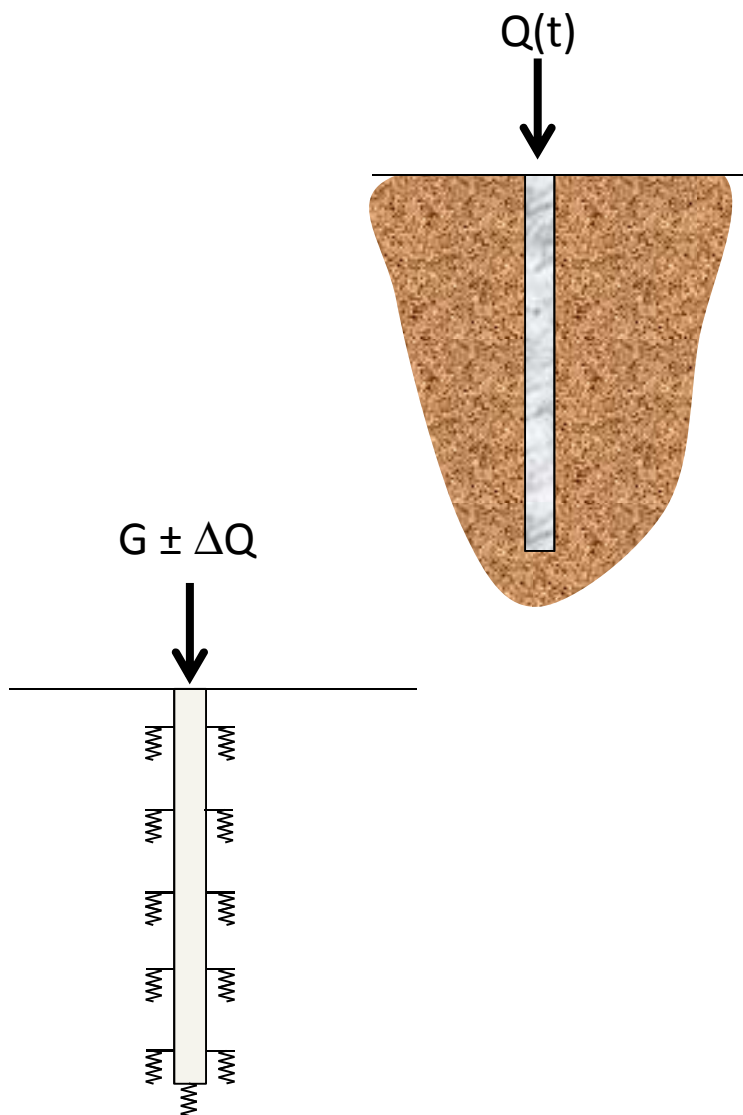
Dimensionnement des pieux sous chargements axiaux

Sébastien Burlon (IFSTTAR)

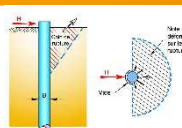
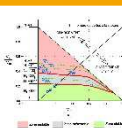
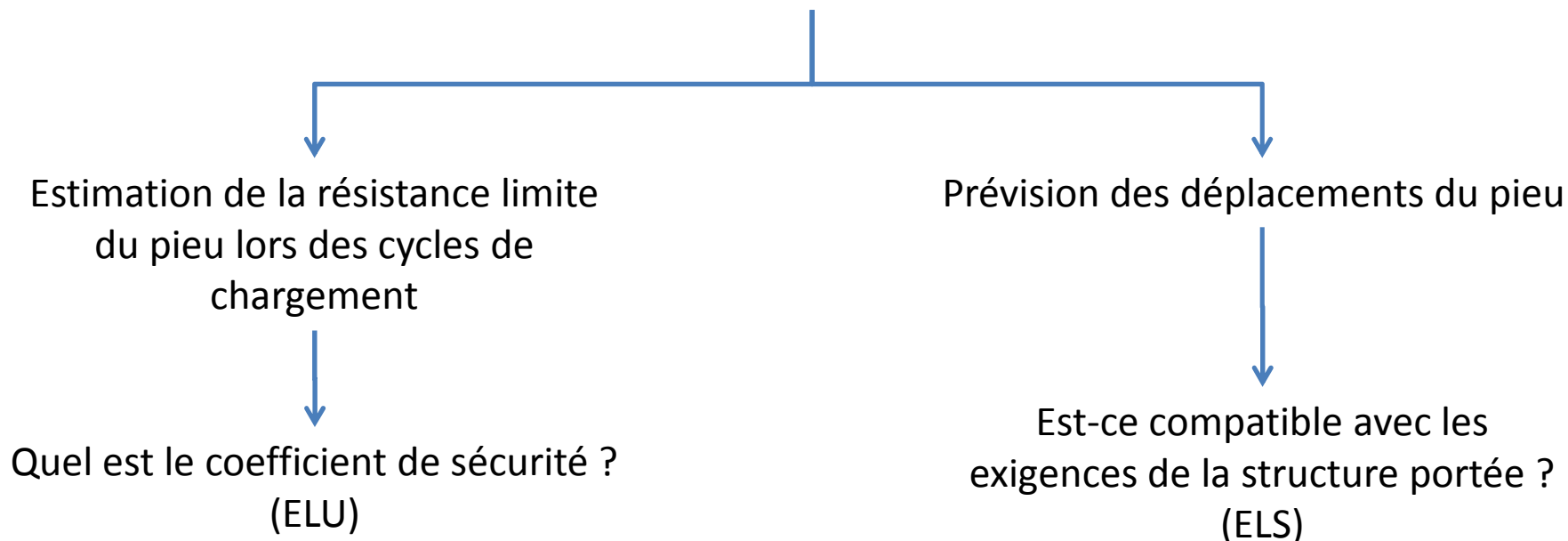
Lundi 13 mars 2017, Paris

SOLCYP – Journée de restitution

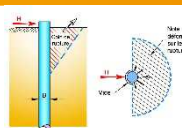
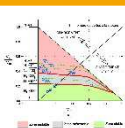
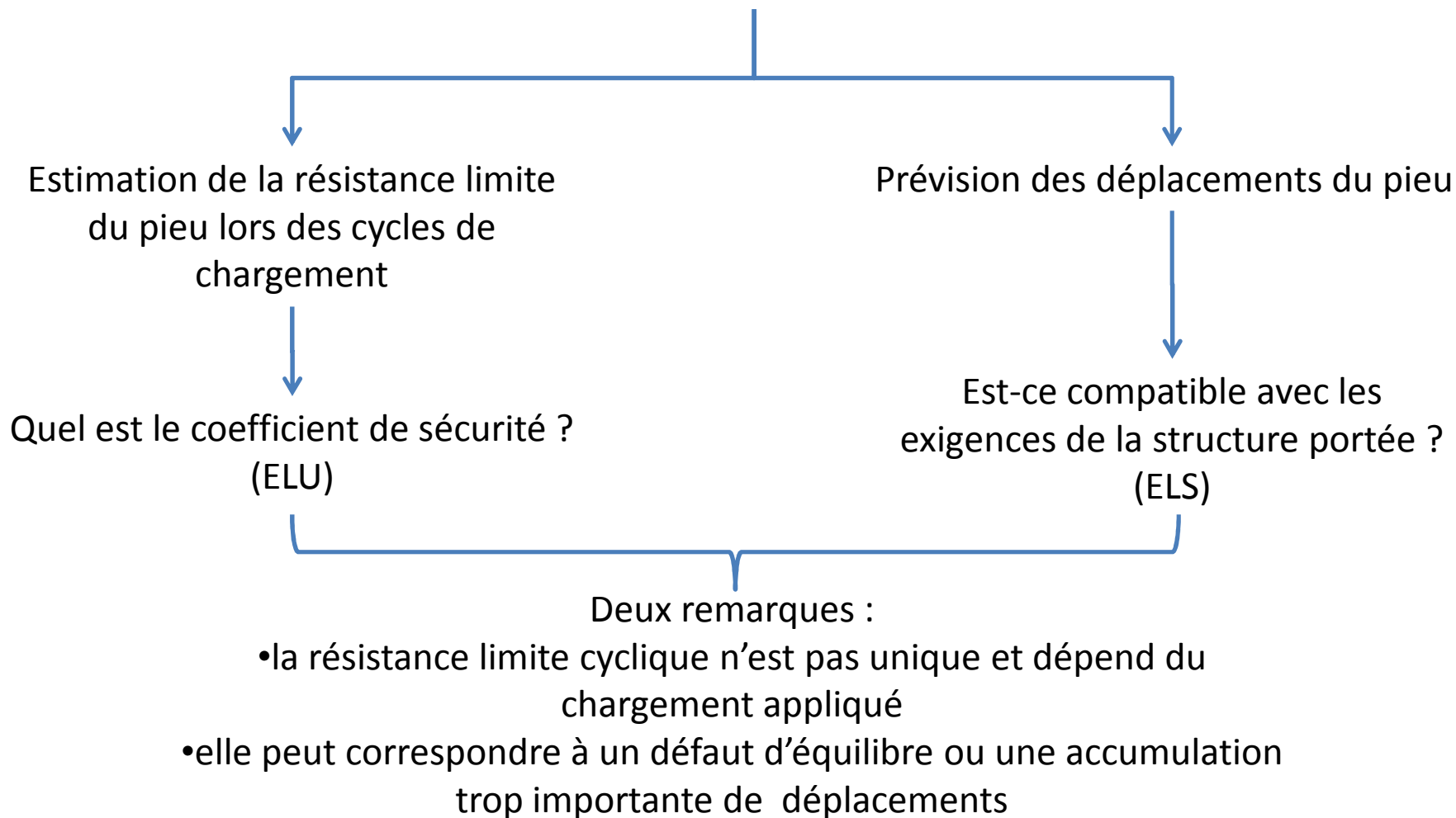




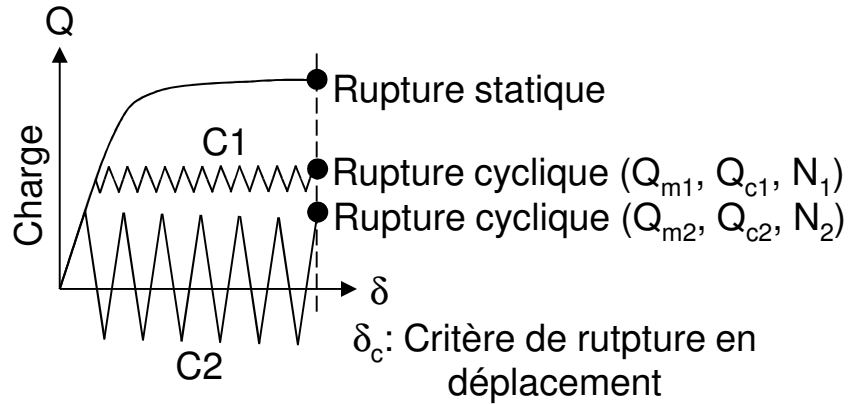
Calcul d'un pieu sous charge axiale cyclique



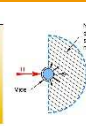
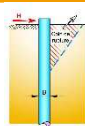
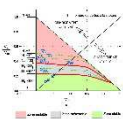
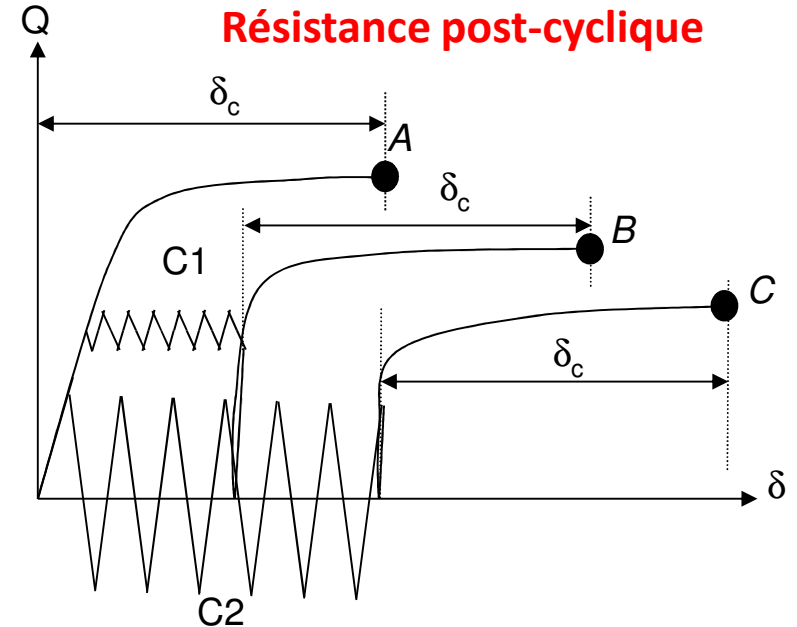
Calcul d'un pieu sous charge axiale cyclique



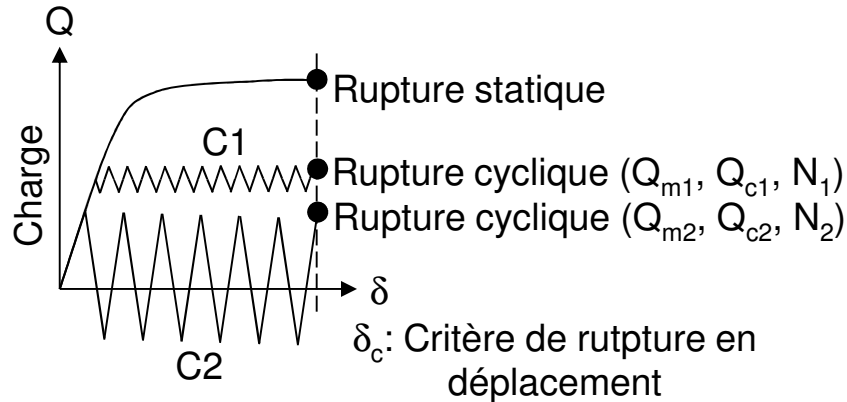
Résistance cyclique



Résistance post-cyclique

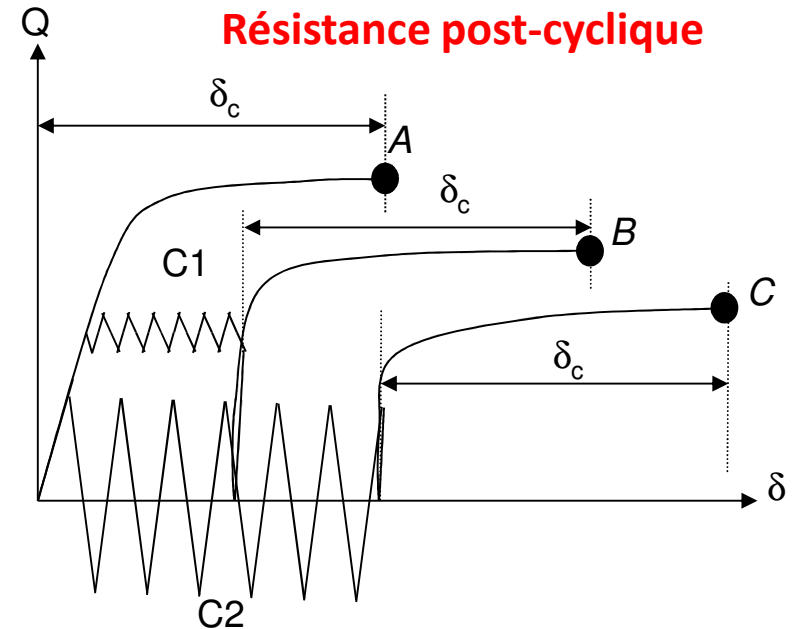


Résistance cyclique

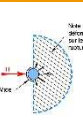
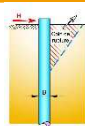
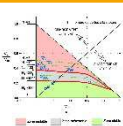


L'objectif des méthodes de calcul développées est de déterminer l'évolution de la résistance cyclique du pieu au cours d'un chargement

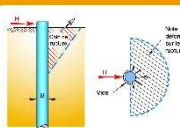
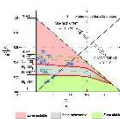
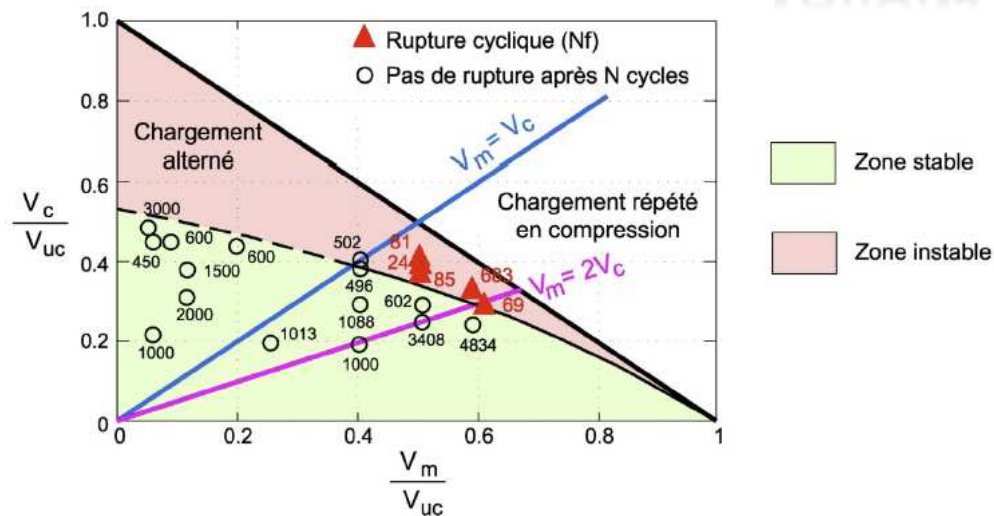
Résistance post-cyclique



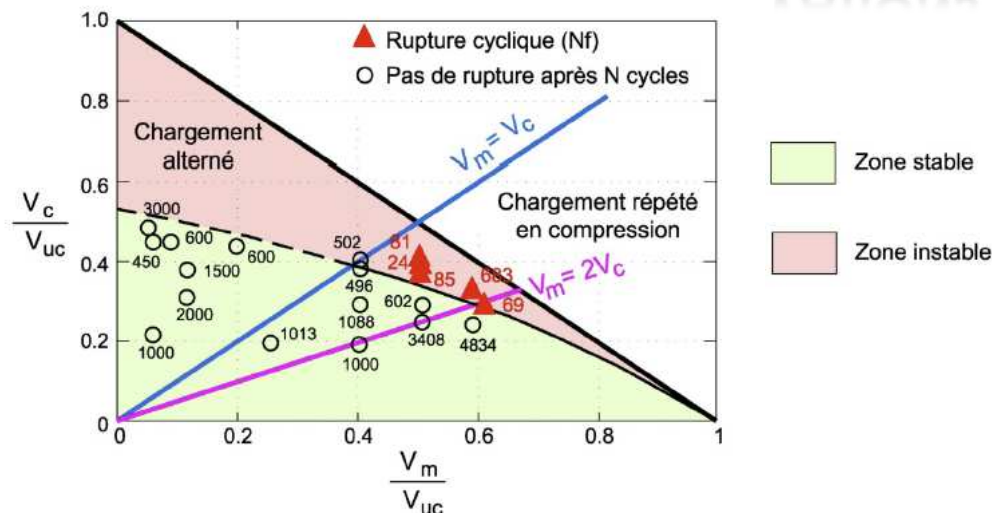
La détermination des résistances post-cycliques est actuellement hors de portée des méthodes développées



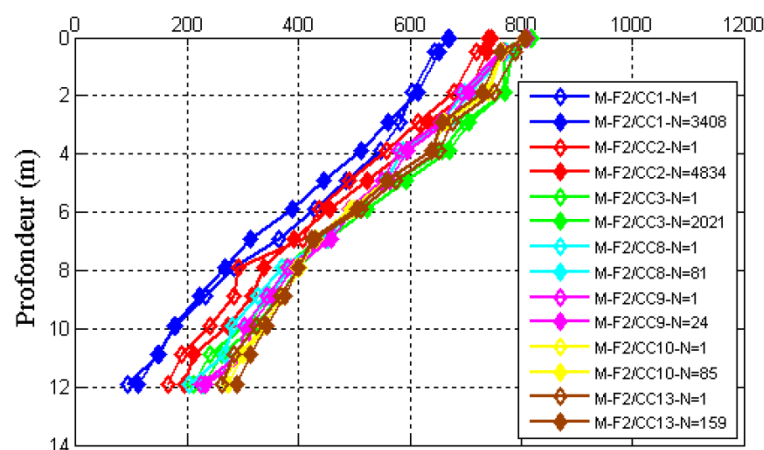
La résistance cyclique dépend du chargement



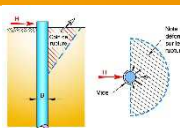
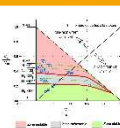
La résistance cyclique dépend du chargement



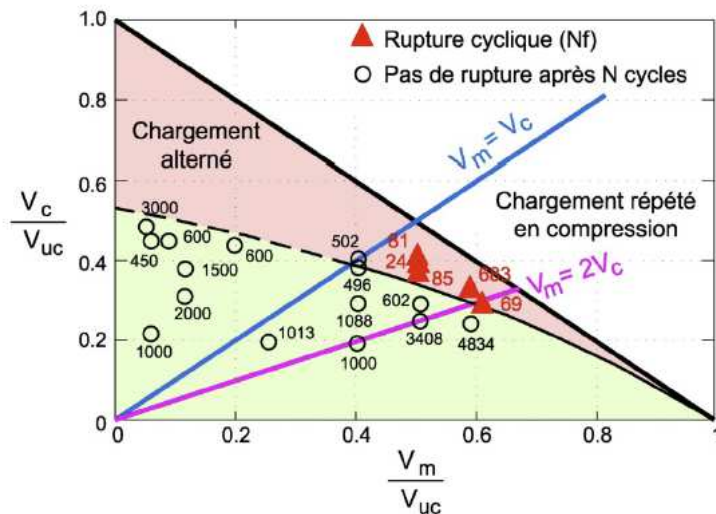
Efforts dans le pieu Q_i (kN)



Diminution du frottement axial limite

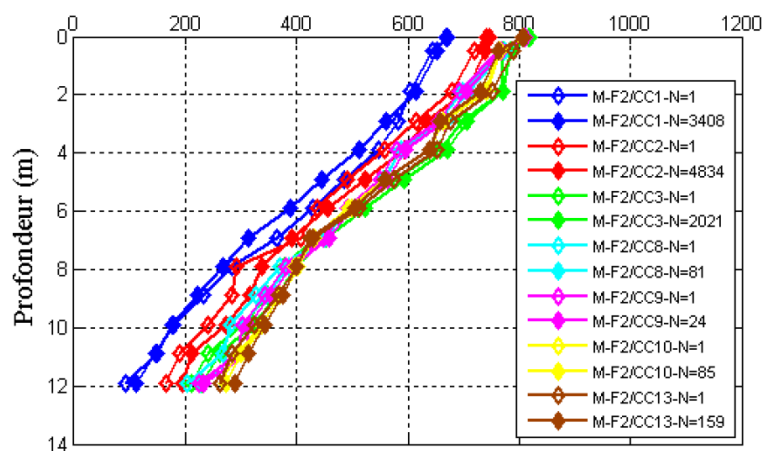


La résistance cyclique dépend du chargement



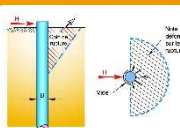
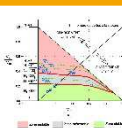
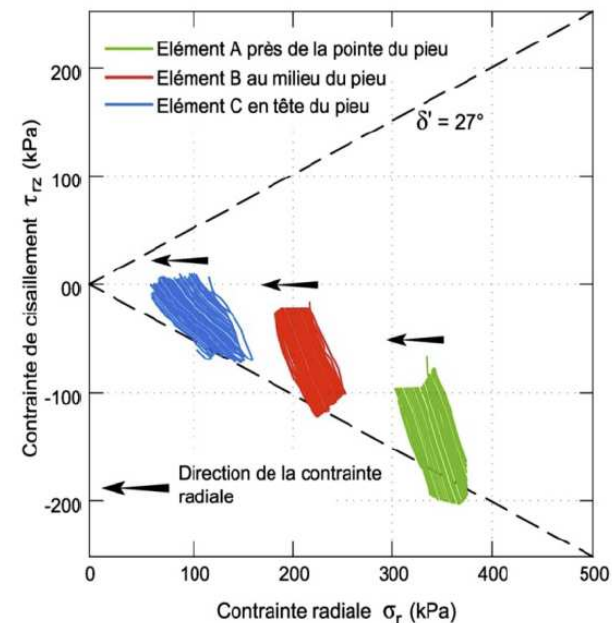
Zone stable
Zone instable

Efforts dans le pieu Q_i (kN)



Diminution du frottement axial limite

Chute de la contrainte radiale effective



Chargement cyclique

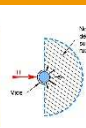
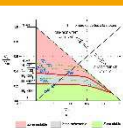


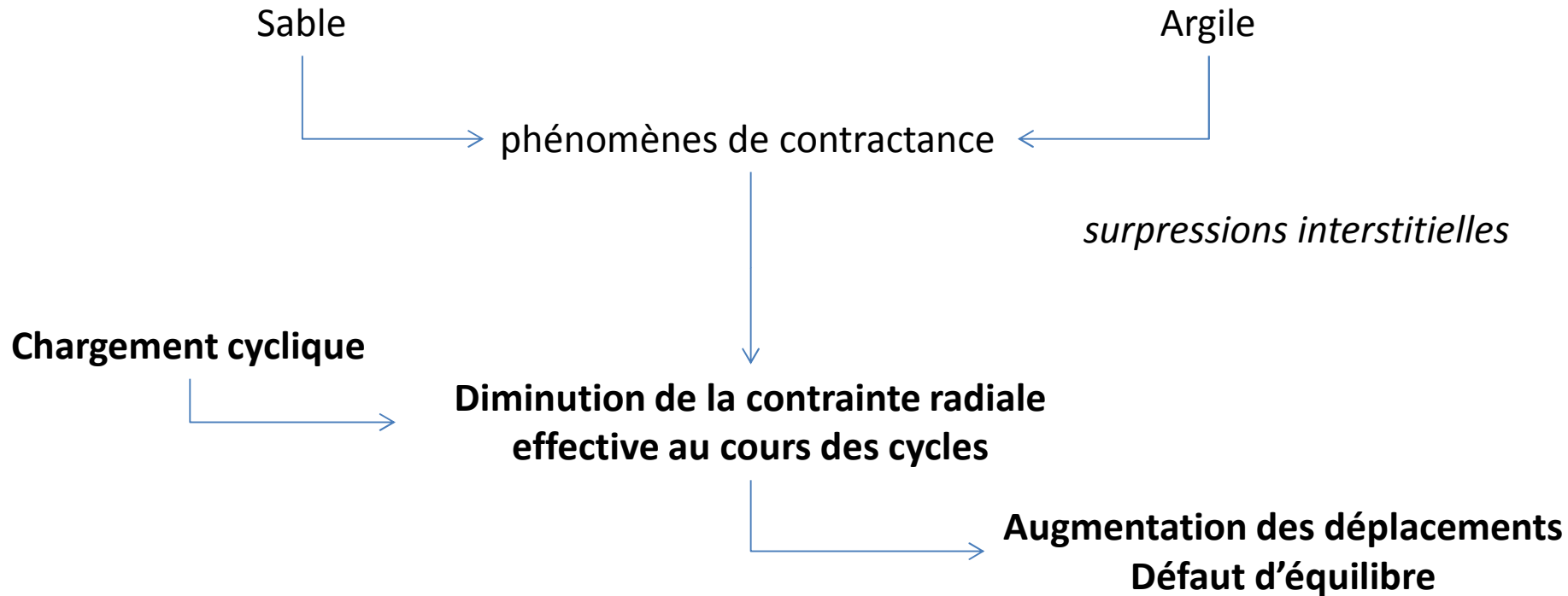
Diminution de la contrainte radiale effective au cours des cycles



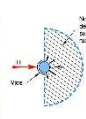
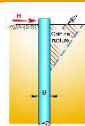
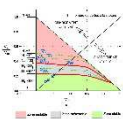
**Augmentation des déplacements
Défaut d'équilibre**

Problèmes non traités : variations de rigidité avec les cycles, effets de la vitesse de chargement, effets du temps, attrition des grains, etc.





Problèmes non traités : variations de rigidité avec les cycles, effets de la vitesse de chargement, effets du temps, attrition des grains, etc.



Méthode t-z enveloppe (ABC, SOLCYP) :

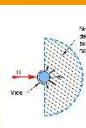
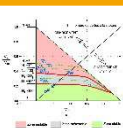
Enjeux : fiabilité de la méthode de dégradation

Méthode des éléments finis ou des différences finies (SOLCYP) :

Enjeux : implémentation d'une méthode d'homogénéisation cyclique, de pseudo-fluage cyclique, de sauts de cycles, etc.

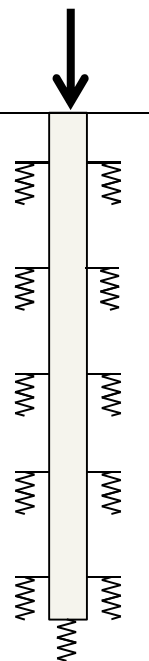
Méthode t-z cycle à cycle (SCARP, RATZ, TZC-ABC, TZC-SOLCYP) :

Enjeux : fiabilité des lois de dégradation, implémentation numérique d'une loi t-z cycle à cycle



Base de données L3SR (Thèse S.Pra-ai, Master G.Pittos)

$G \pm \Delta Q$



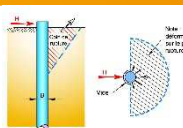
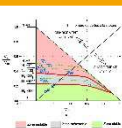
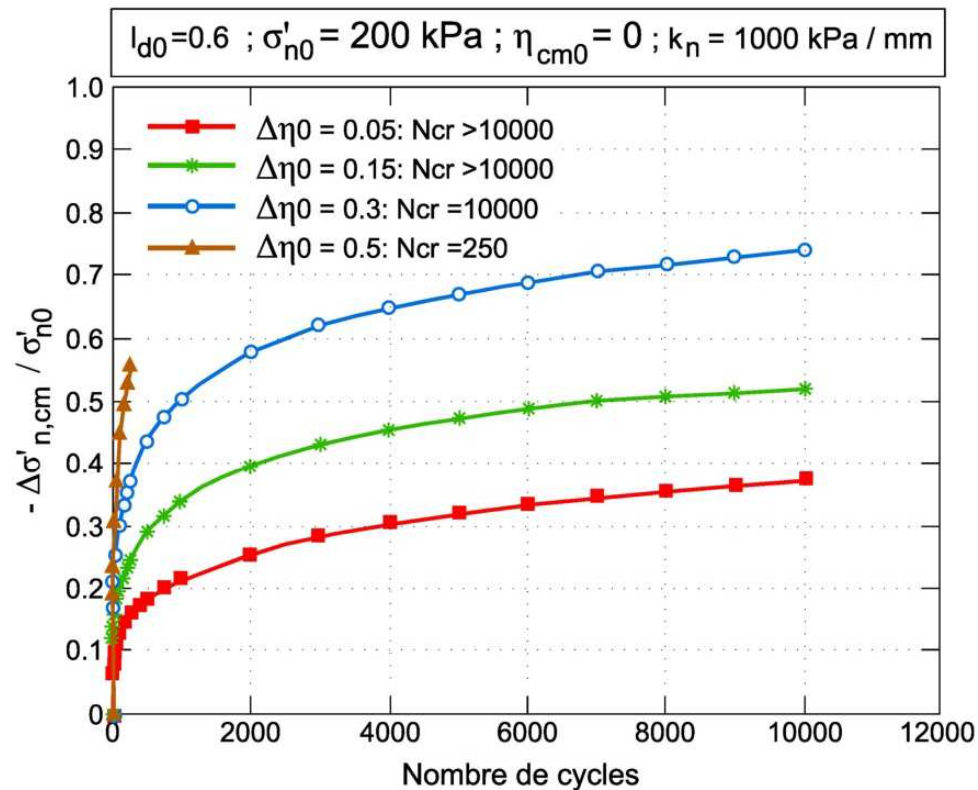
$$\begin{aligned} \Delta\tau \text{ et } \tau_{cm} \\ \rightarrow \Delta\sigma'_n \\ \rightarrow \Delta q_s = \tan\delta \cdot \Delta\sigma'_n \end{aligned}$$

$$\Delta\sigma'_n = K_n [u] \text{ avec } K_n = 2G/R$$

K_n : estimation à partir du pressiomètre

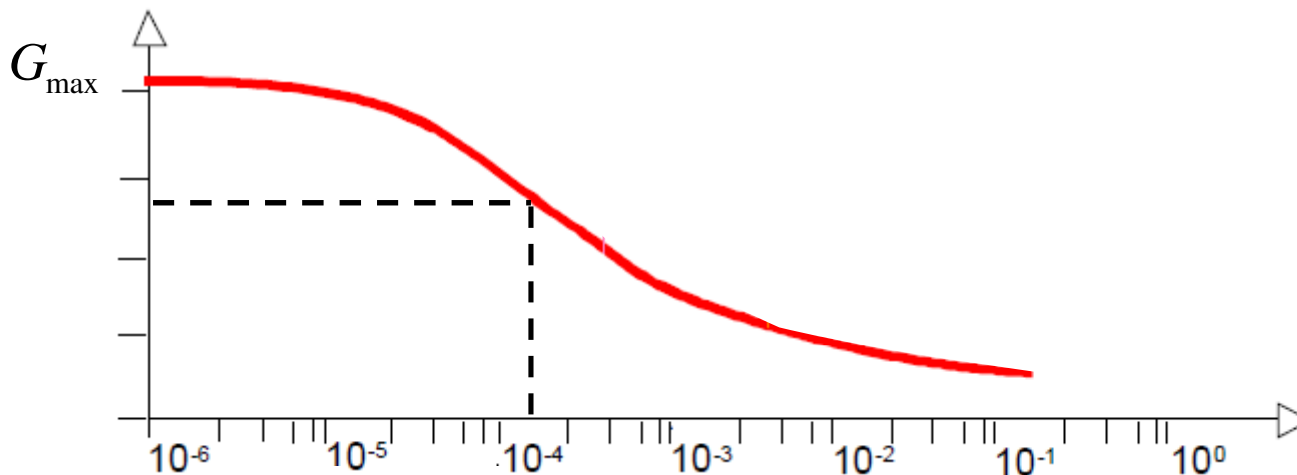
K_n : estimation à partir du pénétromètre

De l'ordre de 200 à 1000 kPa/mm



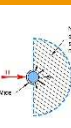
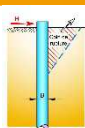
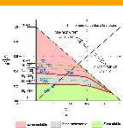
$$K_n = 2G/R$$

→ *Comment estimer le module de cisaillement G ?*

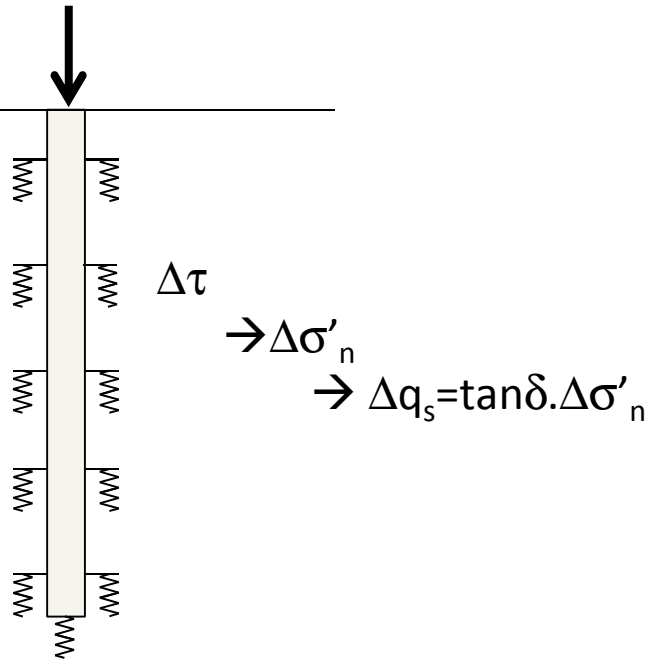


$G_{\text{utilisé}}$ → $G = \frac{G_{\text{max}}}{1,55}$ avec G_{max} calculé avec la formule de *Baldi et al. (1989)*

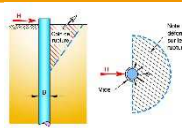
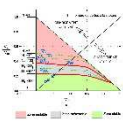
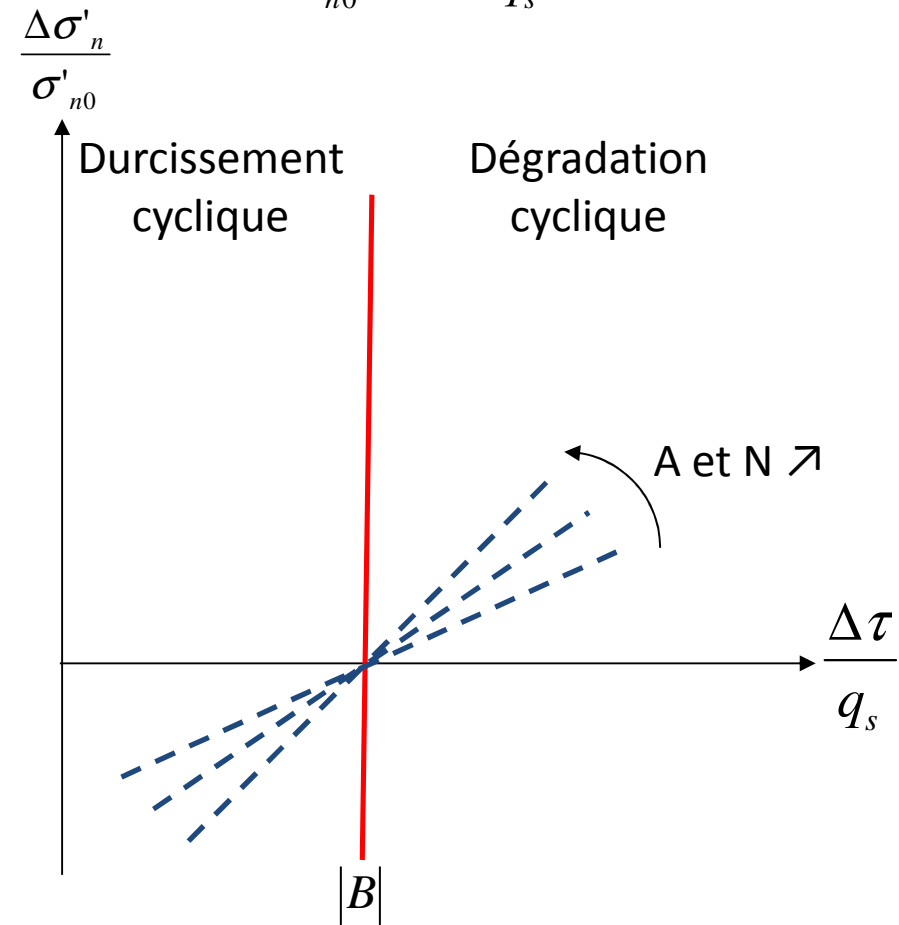
$G_{\text{utilisé}}$ → $G = 10 \left(\frac{E_M}{2(1 + \nu)} \right)$ avec E_M le module pressiométrique



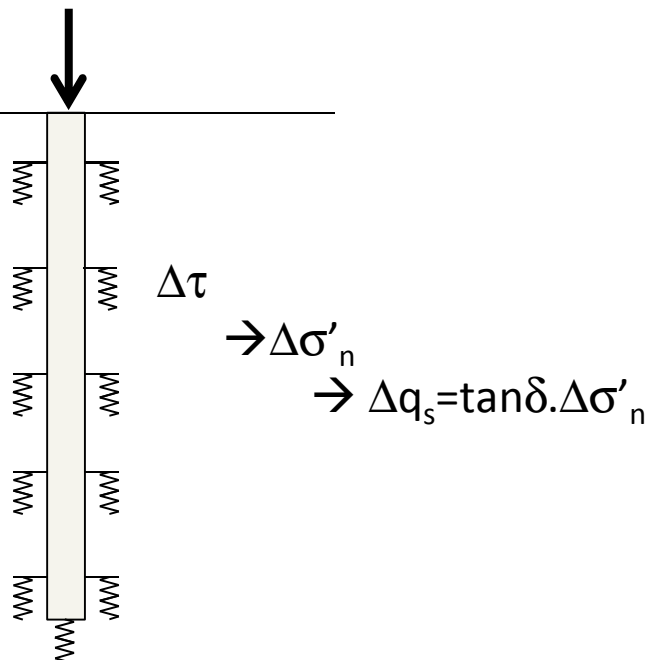
$G \pm \Delta Q$



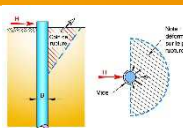
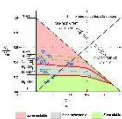
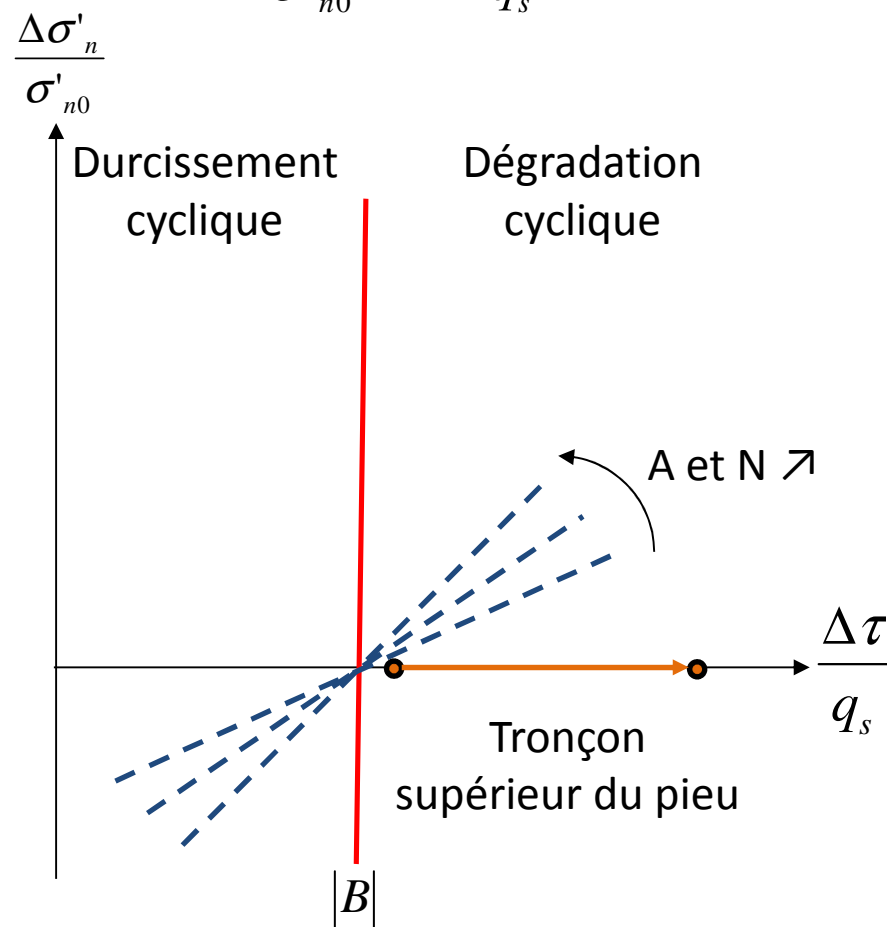
$$\frac{\Delta\sigma'_n}{\sigma'_{n0}} = A \left(\frac{\Delta\tau}{q_s} + B \right) N^c$$



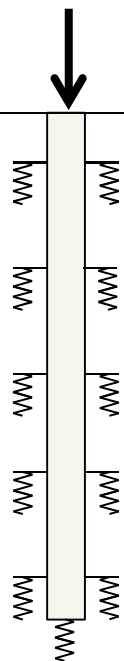
$G \pm \Delta Q$



$$\frac{\Delta \sigma'_n}{\sigma'_{n0}} = A \left(\frac{\Delta \tau}{q_s} + B \right) N^c$$



$G \pm \Delta Q$



$\Delta\tau$

$\rightarrow \Delta\sigma'_n$

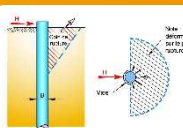
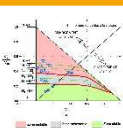
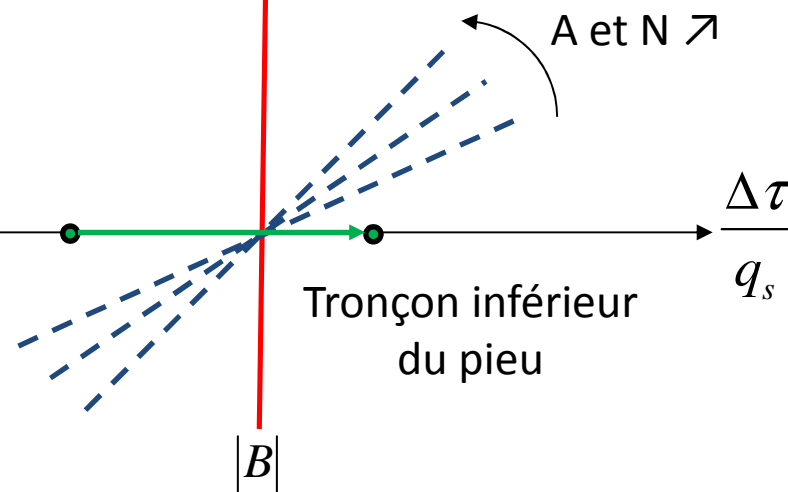
$\rightarrow \Delta q_s = \tan\delta \cdot \Delta\sigma'_n$

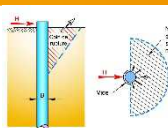
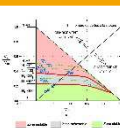
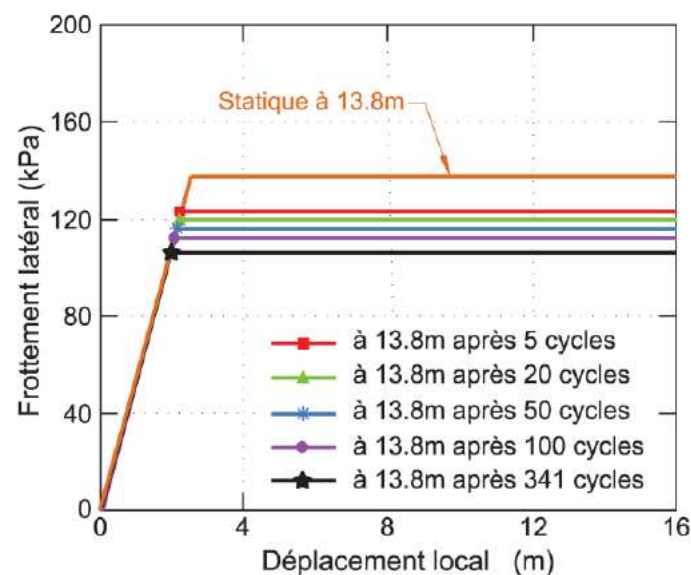
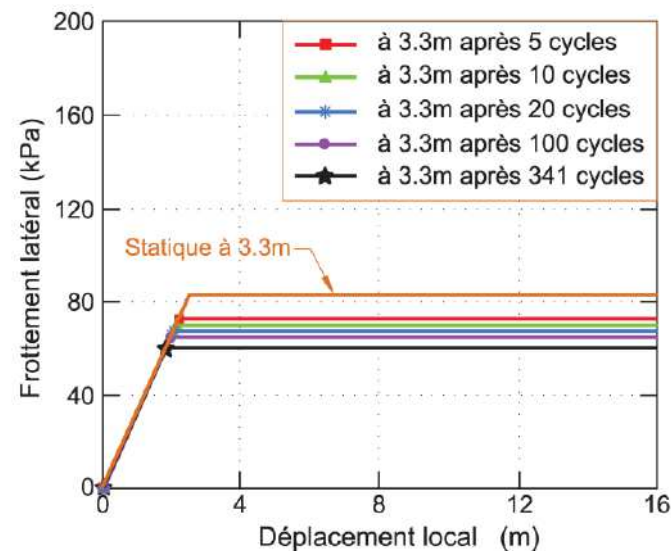
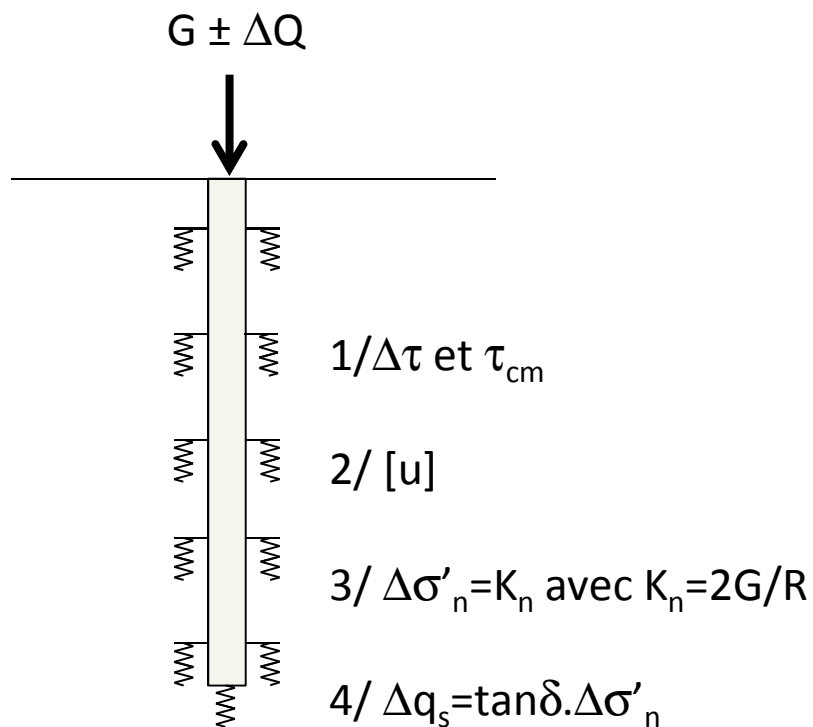
$$\frac{\Delta\sigma'_n}{\sigma'_{n0}} = A \left(\frac{\Delta\tau}{q_s} + B \right) N^c$$

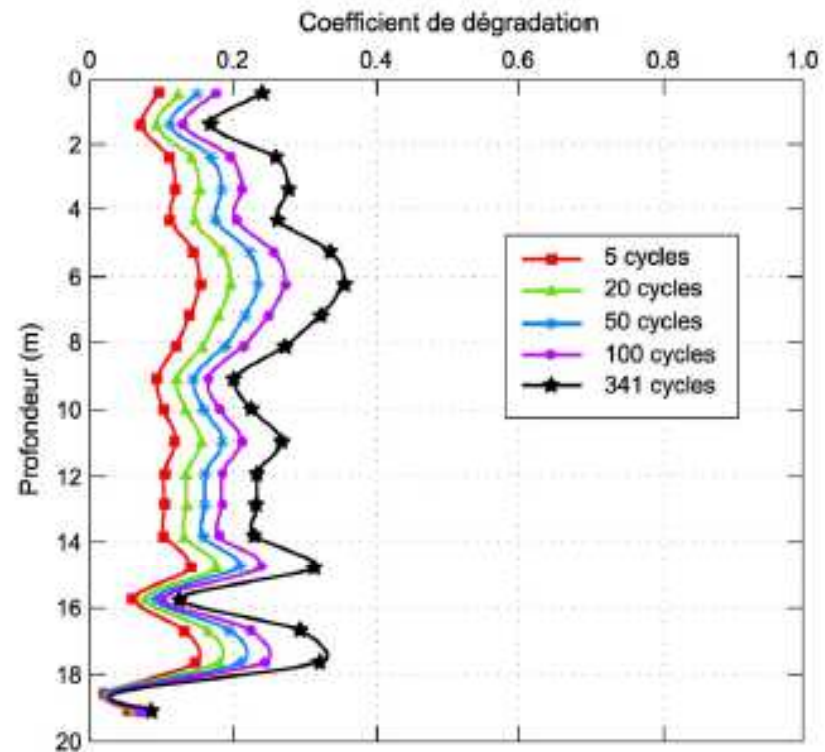
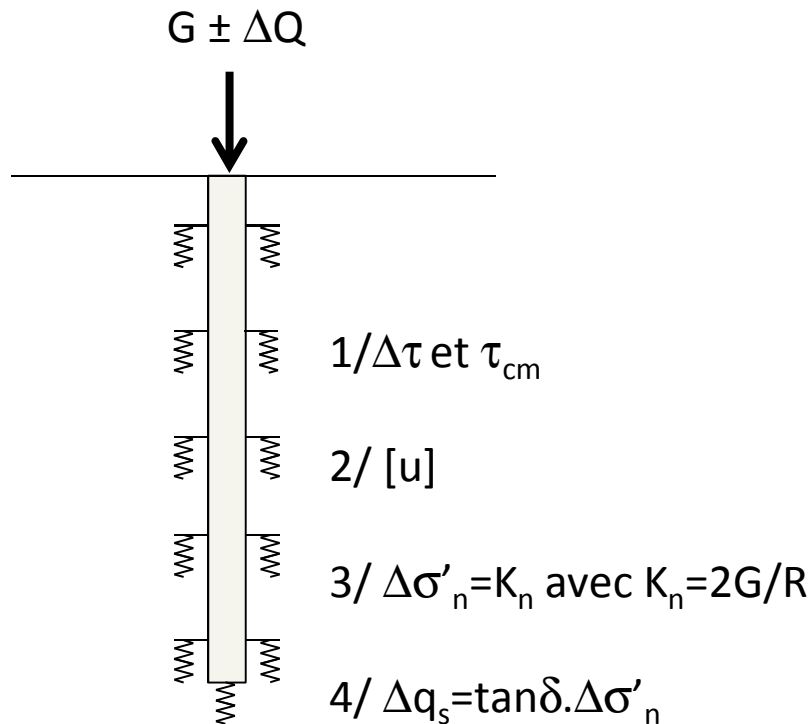
$\frac{\Delta\sigma'_n}{\sigma'_{n0}}$

Durcissement
cyclique

Dégradation
cyclique







- Evolution de la résistance cyclique (actualisation du processus de dégradation au cours des cycles)
- Estimation du déplacement du pieu avec les cycles

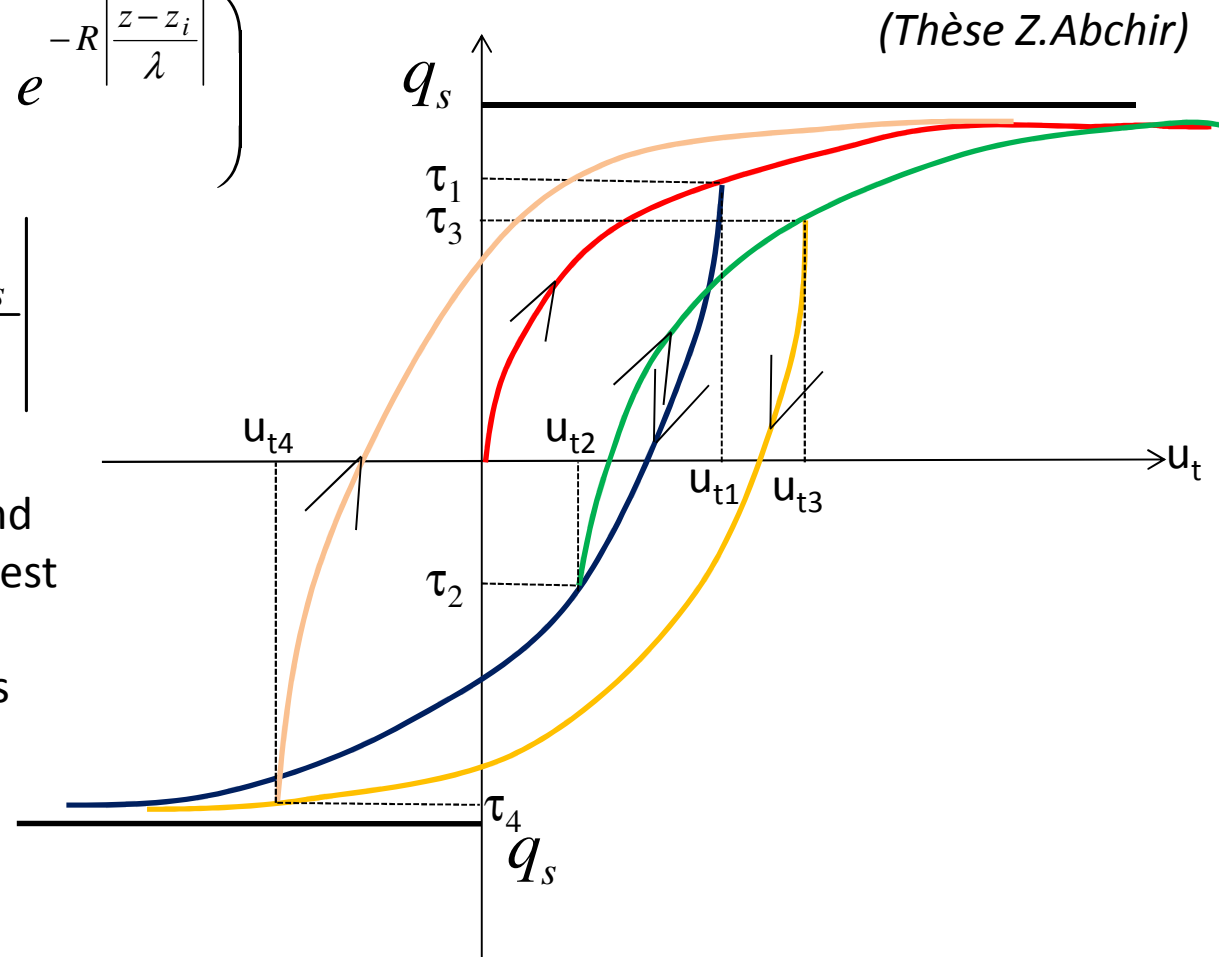


$$\tau = \tau_i + A(-1)^{i+1} q_s \left(1 - e^{-R \left| \frac{z - z_i}{\lambda} \right|} \right)$$

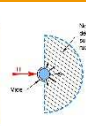
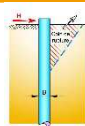
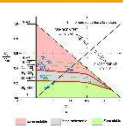
$$A = \left| \frac{\tau_i - (-1)^{i+1} q_s}{q_s} \right|$$

La raideur en décharge dépend de la contrainte atteinte, elle est multipliée par 1 à 2.

Elle peut évoluer au cours des cycles avec le paramètre R.



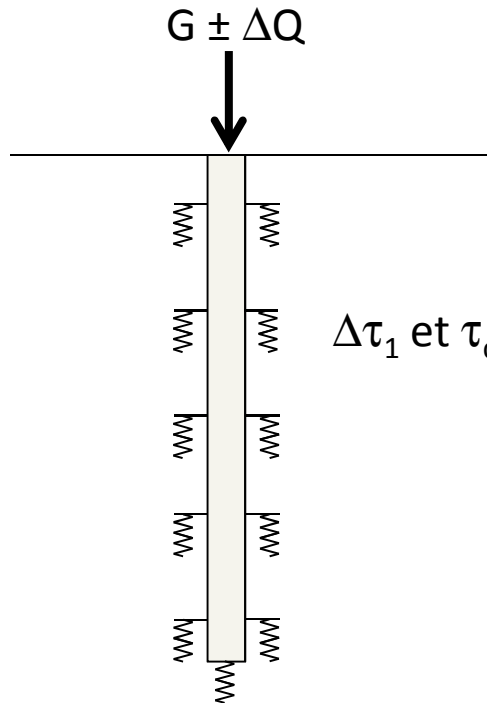
→ *Comment varie le frottement axial au cours des cycles ?*



- La variation de frottement axial est associée à une variation de la contrainte normale :

$$\Delta q_s(i) = \tan(\delta) * \Delta \sigma'_n$$

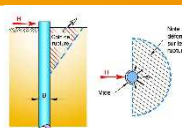
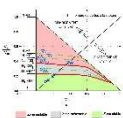
- Un premier cycle de chargement est appliqué pour déterminer à chaque profondeur les variations de contraintes: $\Delta \tau_1$ et τ_{cm1} .



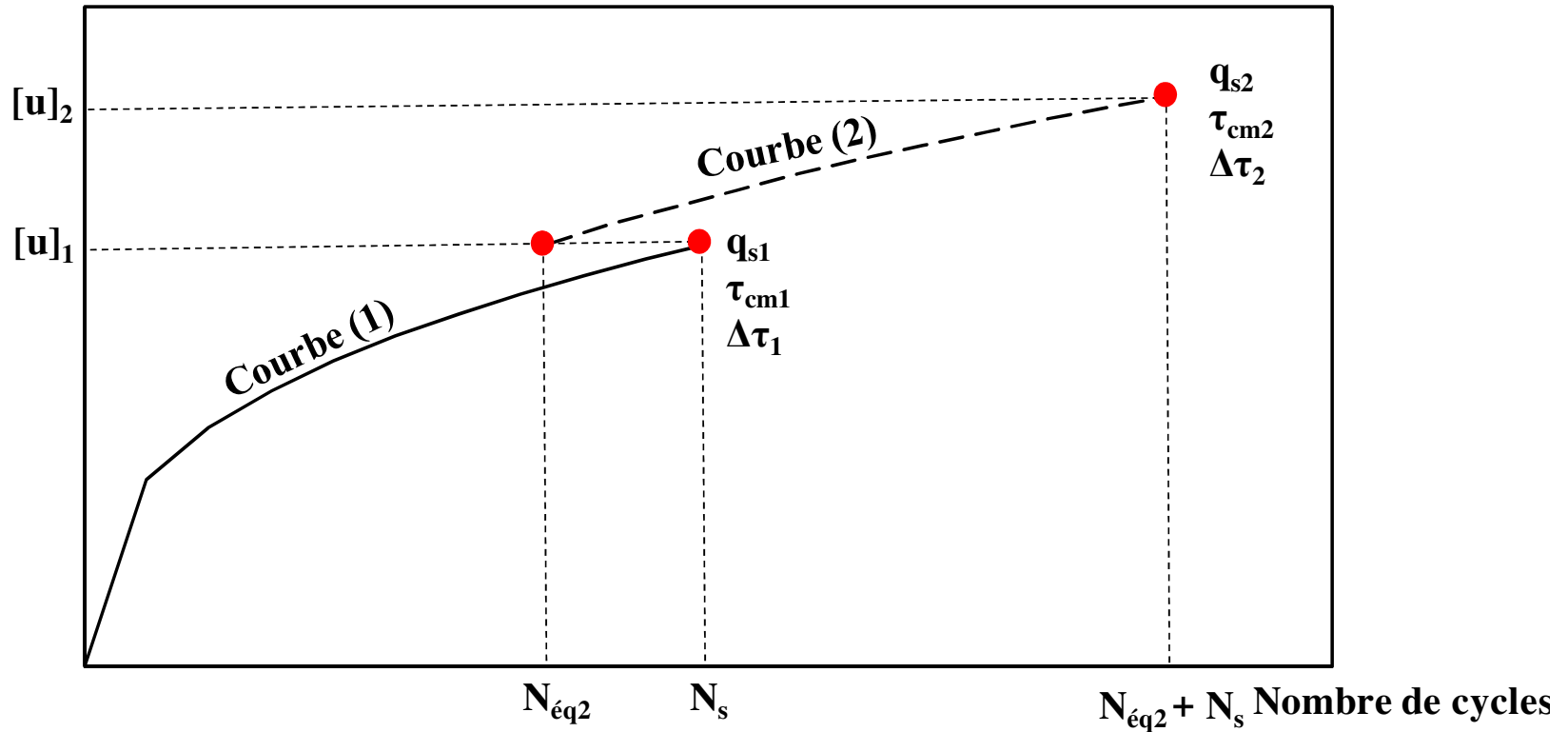
$\Delta \tau_1$ et $\tau_{cm1} \rightarrow$ estimation de la variation du déplacement normal $[u]$ à l'interface sol-pieu
 $\rightarrow$ variation de la contrainte normale σ'_n à l'interface sol-pieu :

$$\Delta \sigma'_n = K_n [u] \text{ avec } K_n = 2G/R$$

Voir base de données L3SR

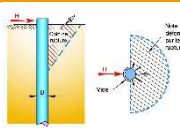
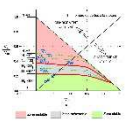


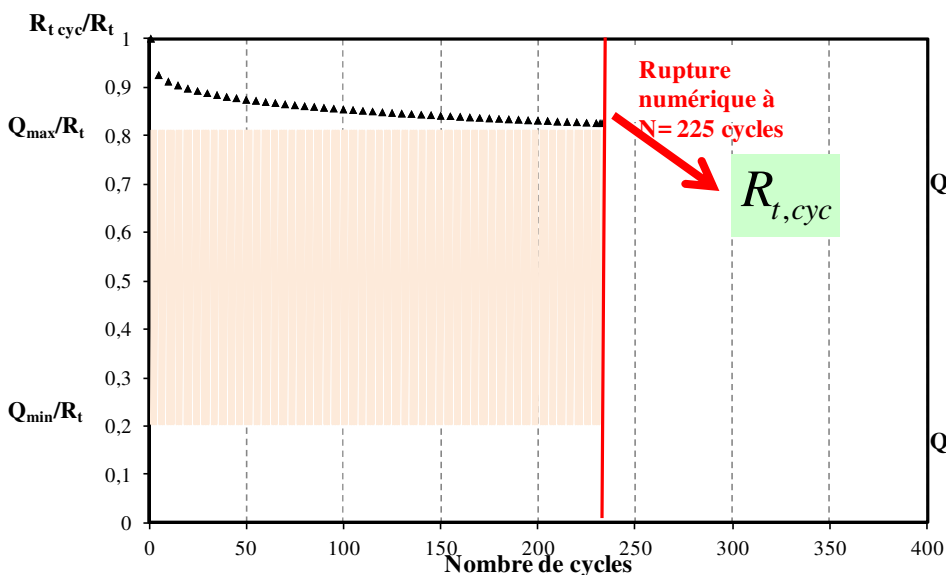
Déplacement
normal relatif [u]



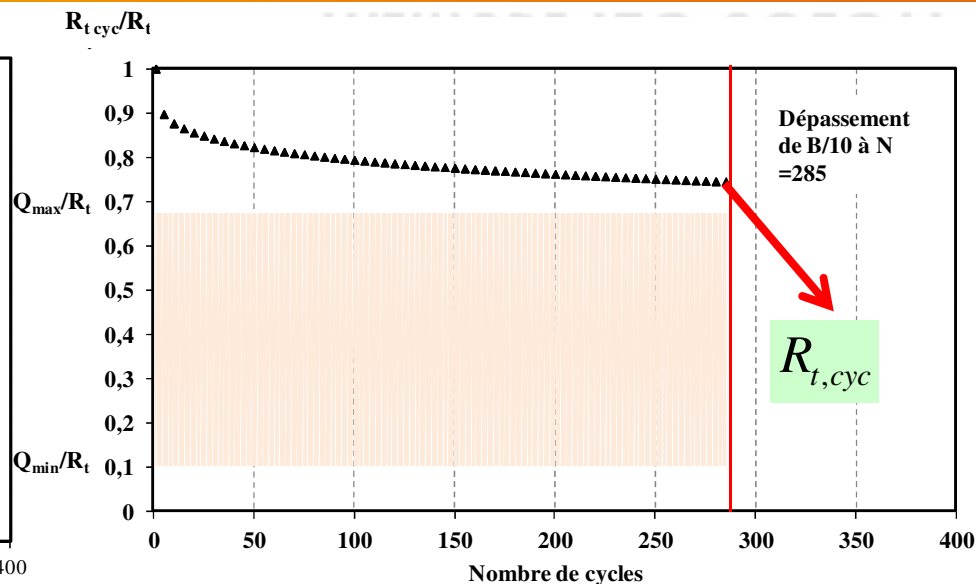
Actualisation des courbes de dégradation :

$$f_1(\tau_{cm1}, \Delta\tau_1, N_{ég1} + N_s) = f_2(\tau_{cm2}, \Delta\tau_2, N_{ég2})$$





Mesure : 345 cycles



Mesure : 367 cycles

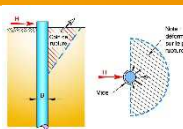
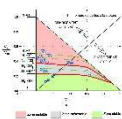
Justifications à réaliser sur un cas réel (dans un format LRFD) :

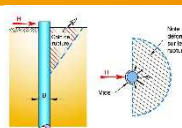
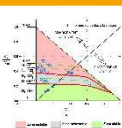
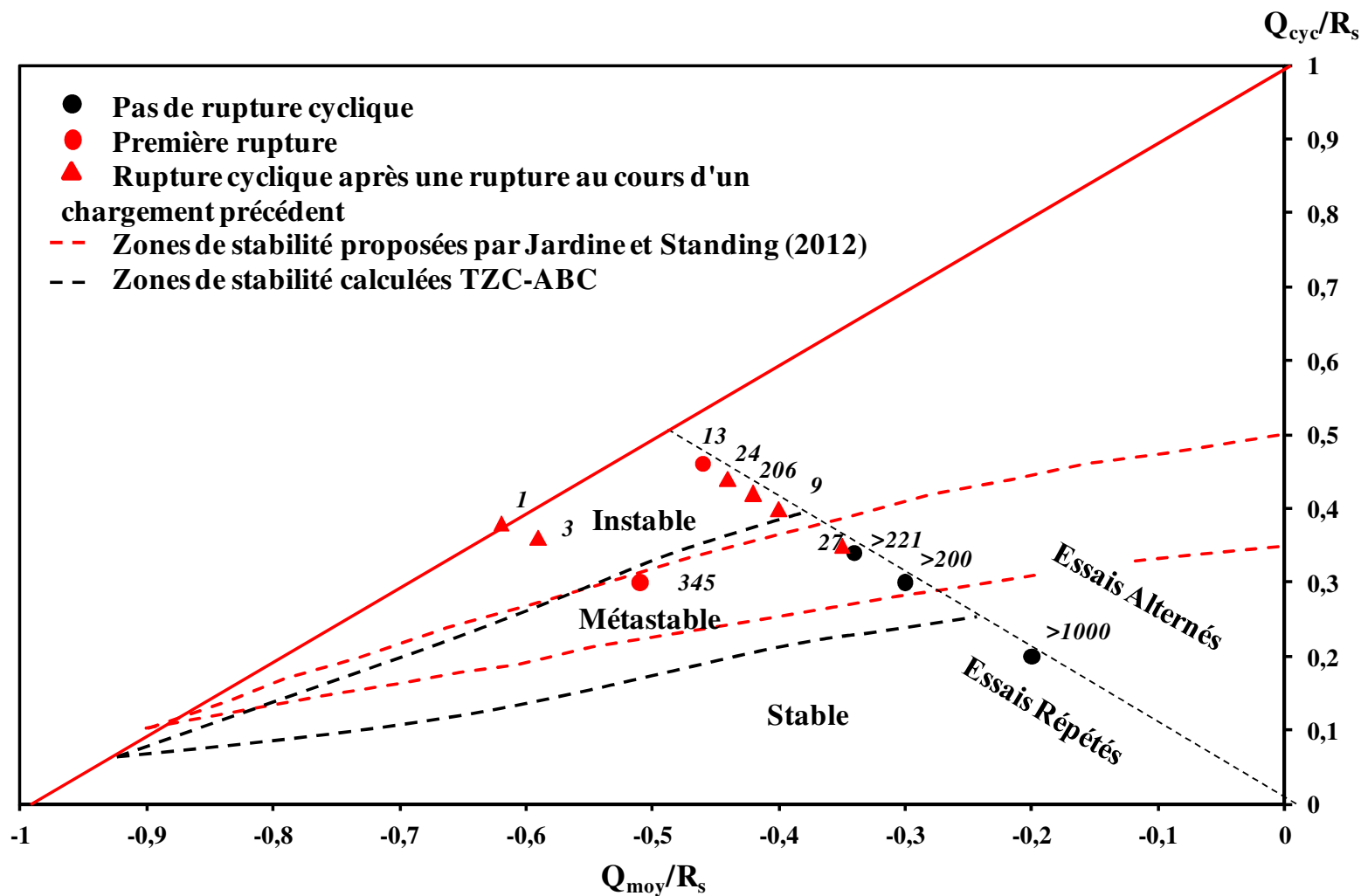
$$\gamma_G G_k + \gamma_Q Q_k < \frac{R_{t,cyc}}{\gamma_R}$$

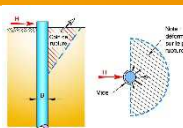
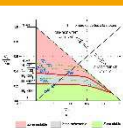
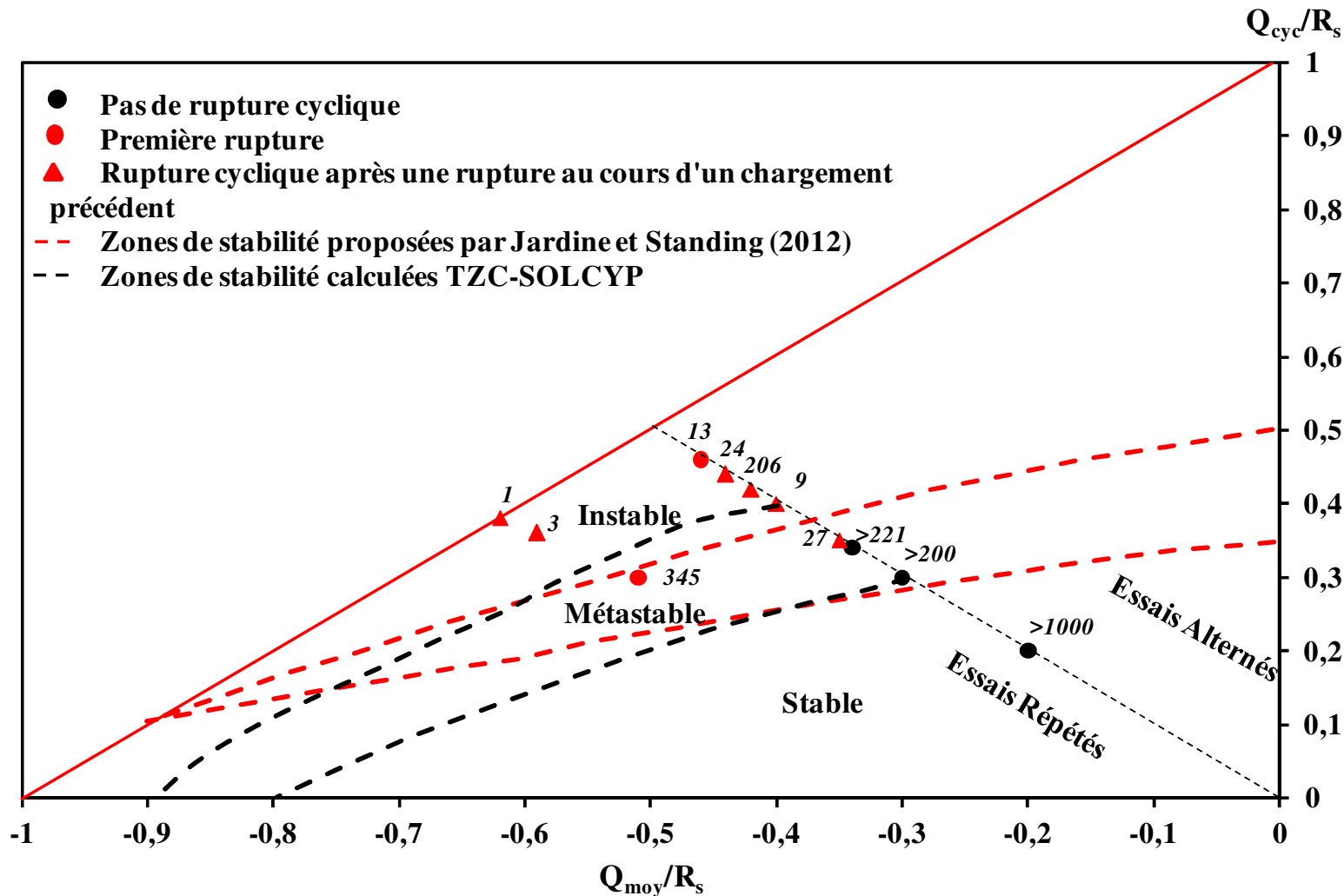
En traction

$$\gamma_G G_k + \gamma_Q Q_k < \frac{R_{c,cyc}}{\gamma_R}$$

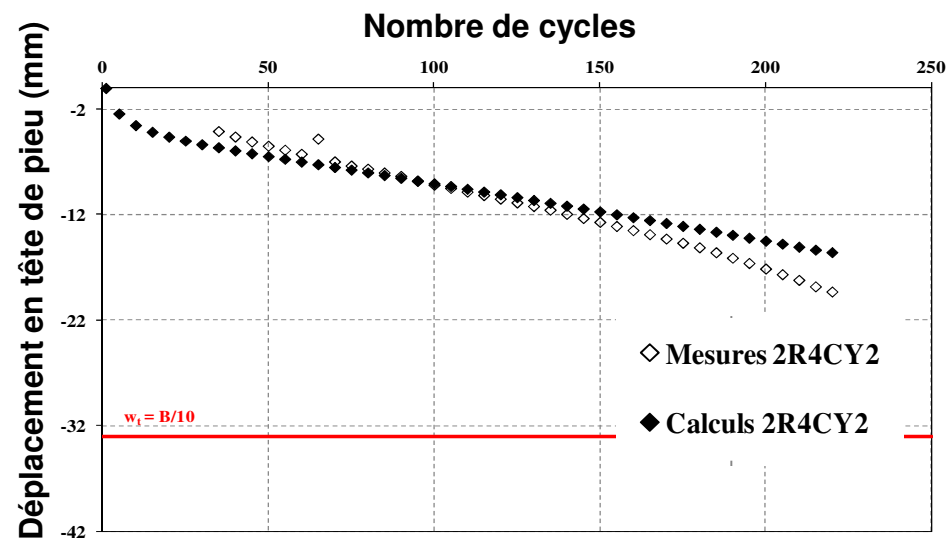
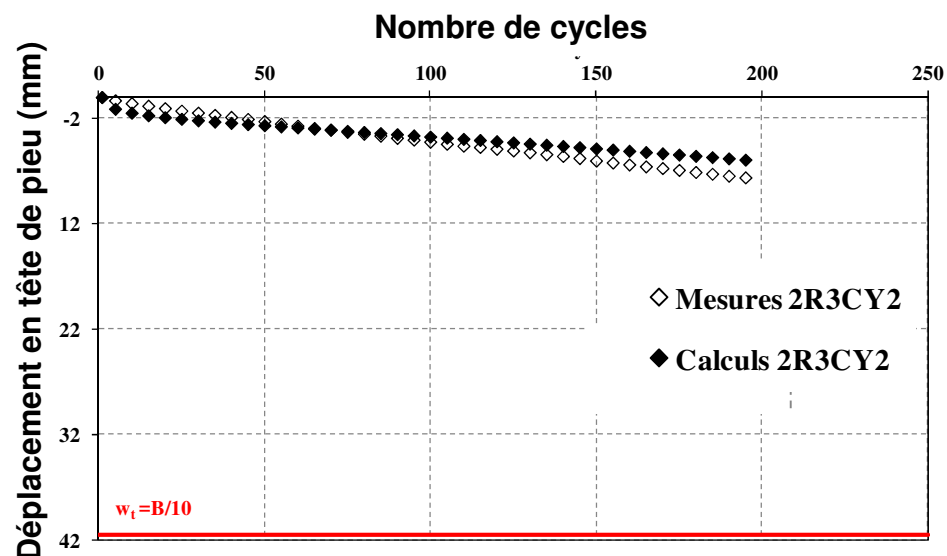
En compression



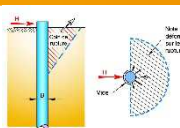
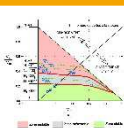


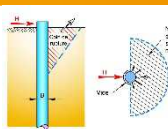
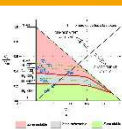
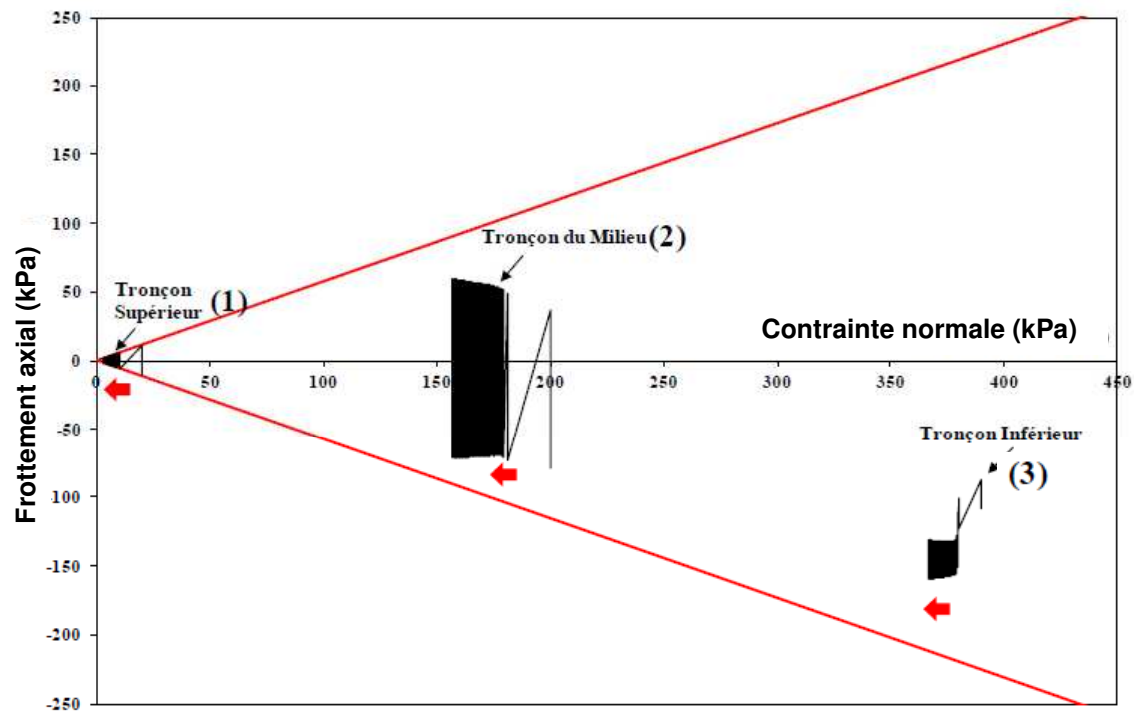
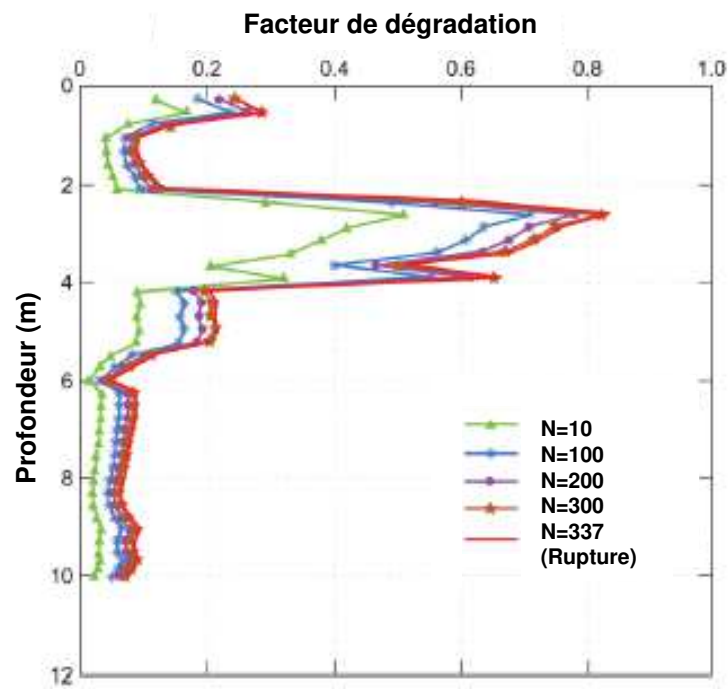


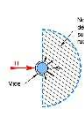
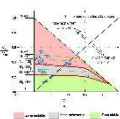
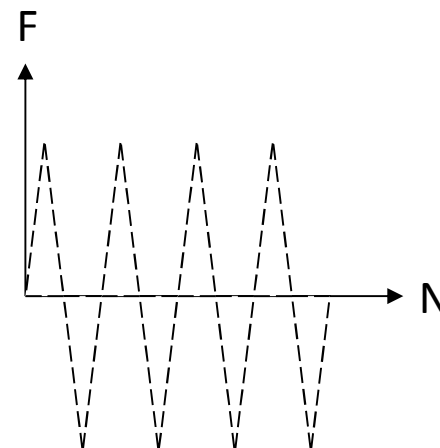
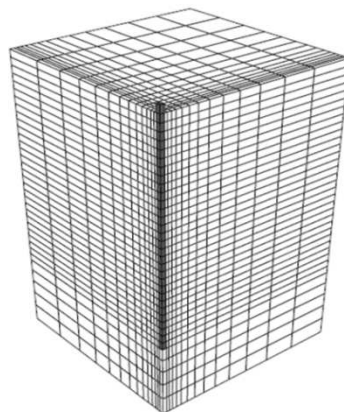
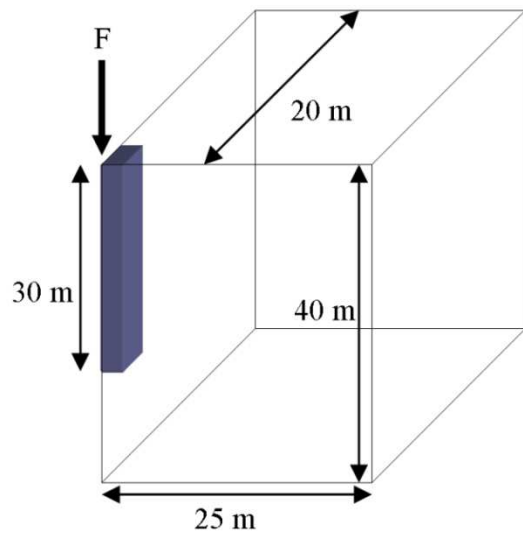
Sable, Dunkerque, GOPAL

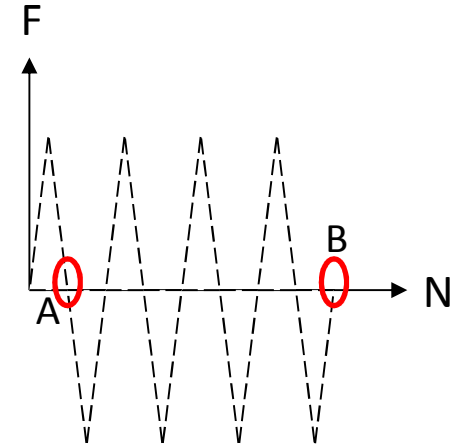
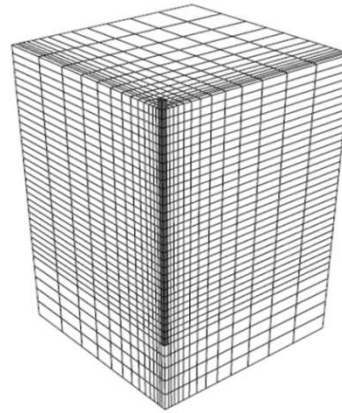
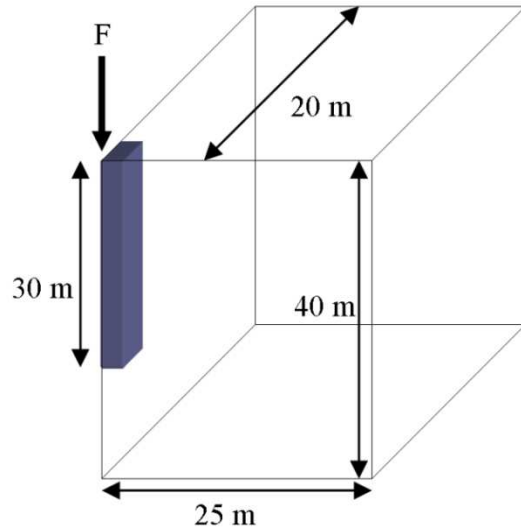


La méthode développée fonctionne bien tant que le pieu ne présente pas un changement de comportement : d'une zone stable ou métastable vers une zone instable.

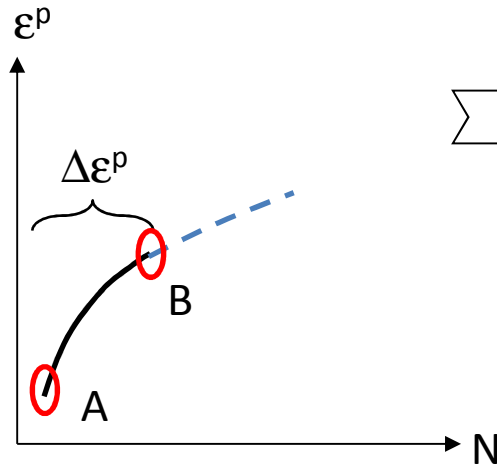






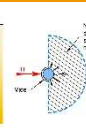
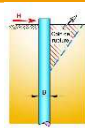
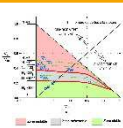


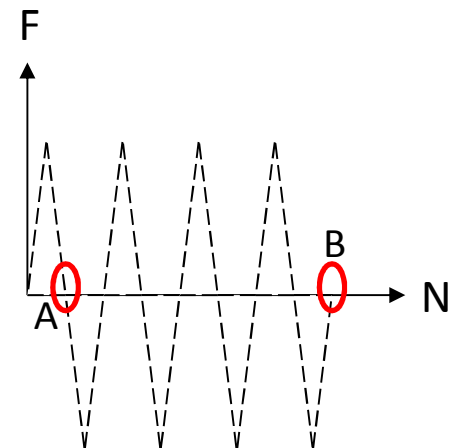
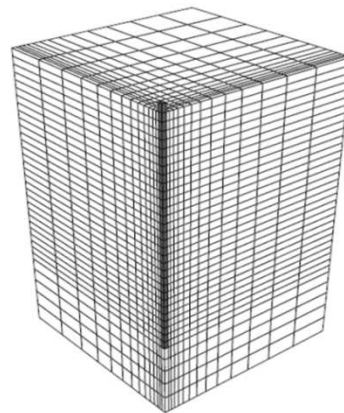
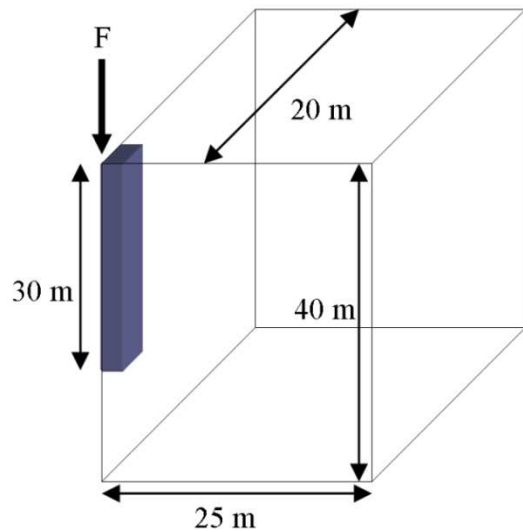
Approche numérique



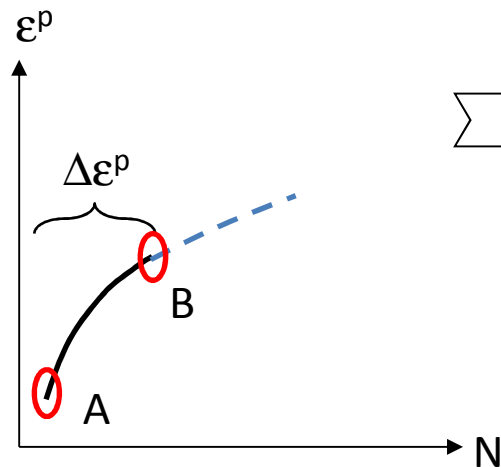
$$\Rightarrow \Delta F = \int^t BC \Delta \varepsilon^p d\Omega$$

(Vecteur de force nodale)





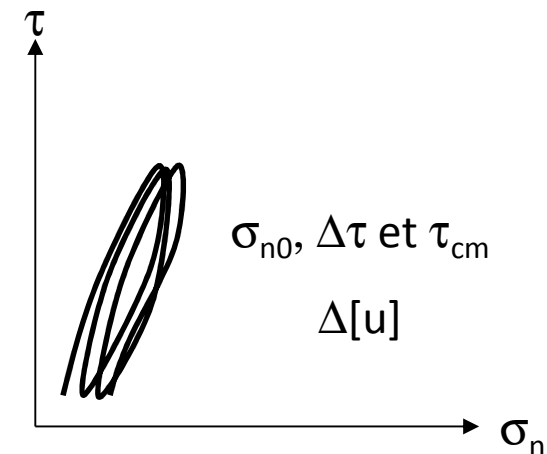
Approche numérique

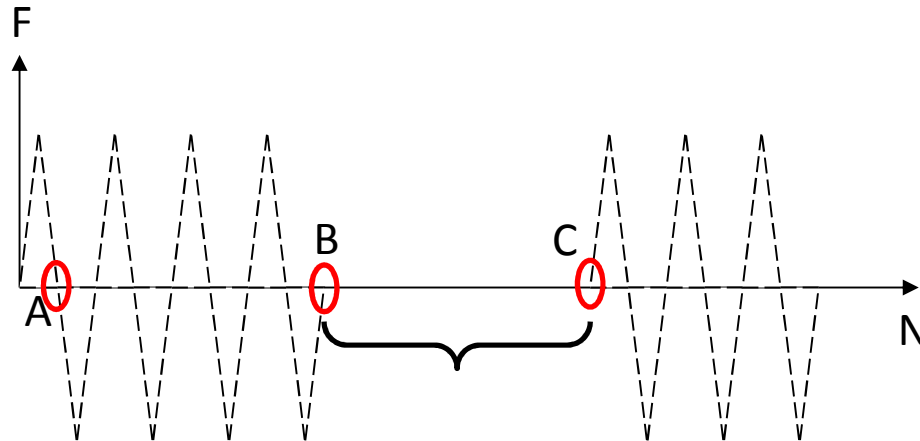


$$\Rightarrow \Delta F = \int^t BC \Delta \varepsilon^p d\Omega \Leftarrow$$

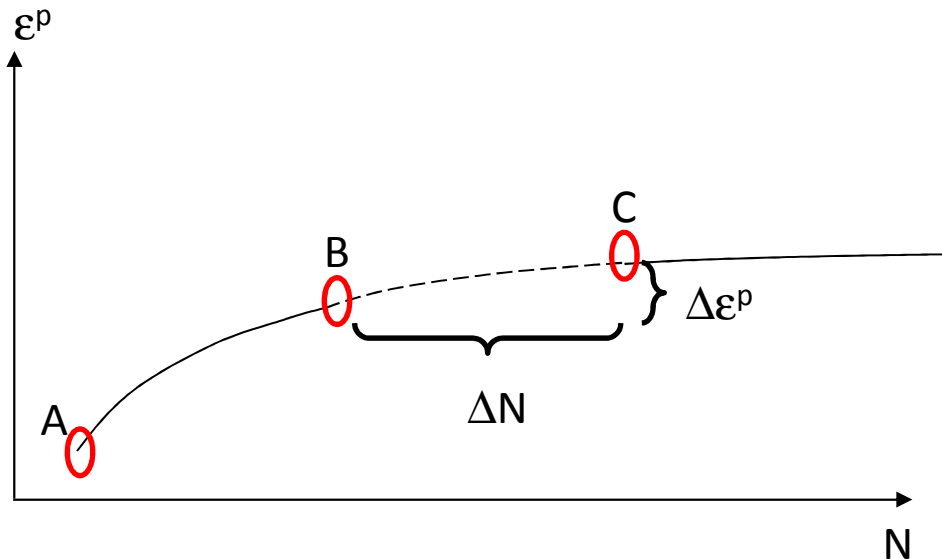
(Vecteur de force nodale)

Approche numérique et expérimentale



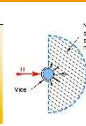
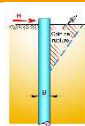
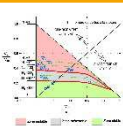


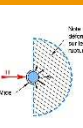
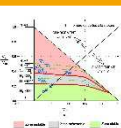
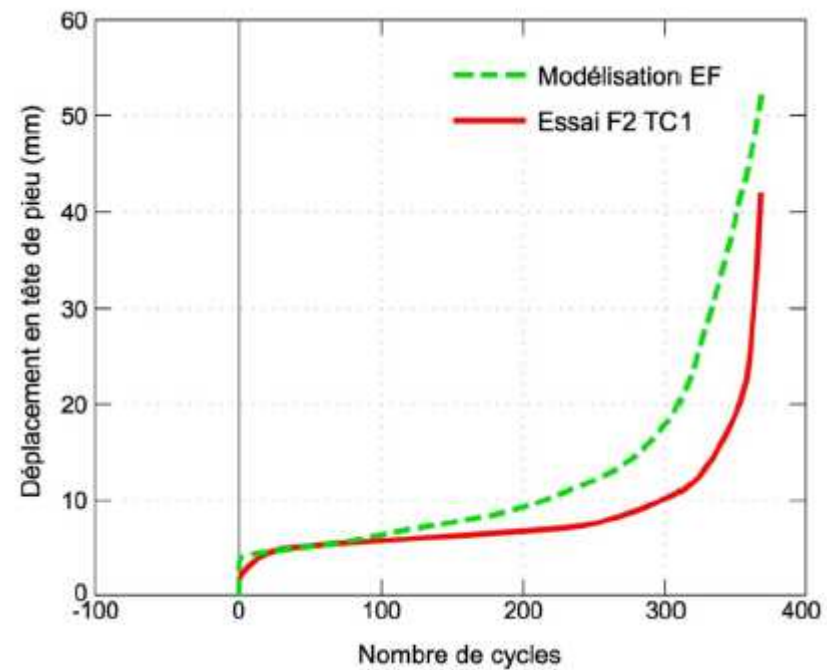
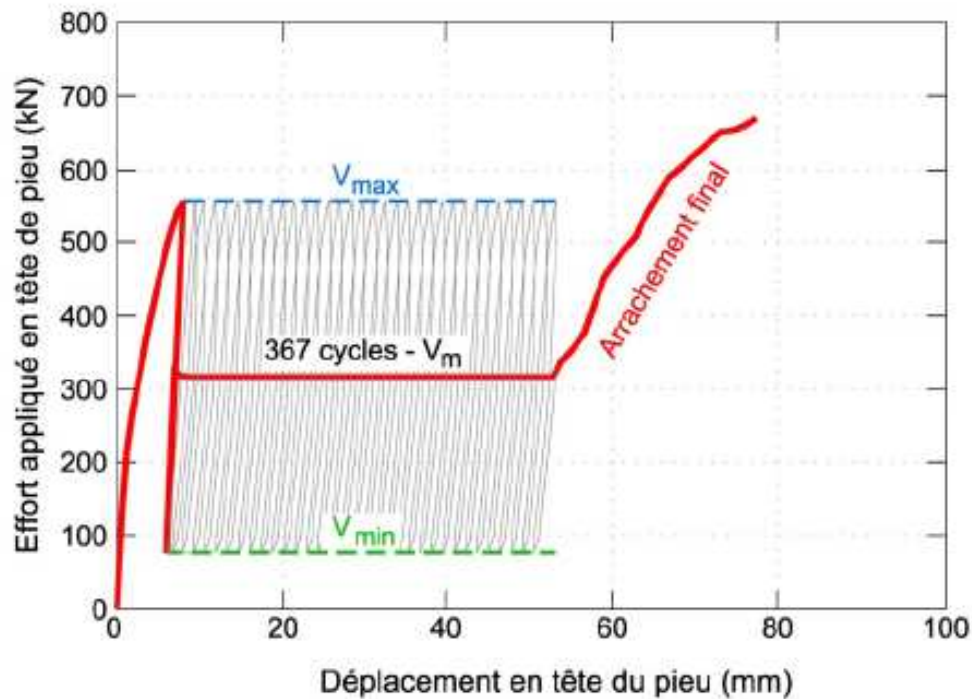
Le chargement global appliqué sur les cycles précédents est nul mais des formations plastiques apparaissent



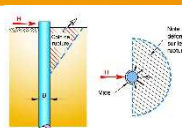
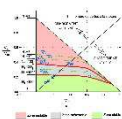
$$\Rightarrow \Delta F = \int^t BC \Delta \varepsilon^p d\Omega$$

(Vecteur de force nodale)

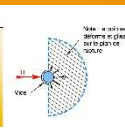
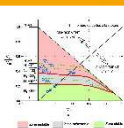




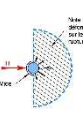
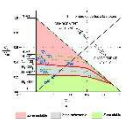
Méthodes de calcul	Disponibilités des données	Robustesse du calcul	Fiabilité du calcul	Temps de calcul	Facilité d'utilisation
t-z enveloppe	++	++	-/+	+	+
t-z cycle à cycle	++	+	+	+	+
méthode des éléments finis ou des différences finies	-	-/+	-/+	-	-/+



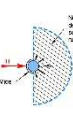
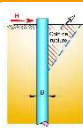
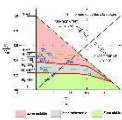
- L'estimation de la capacité portante cyclique reste une tâche complexe pour laquelle des solutions raisonnables existent avec divers degrés de difficultés.
- Le calcul des déplacements de pieux soumis à des charges cycliques reste malgré tout délicat. Divers points restent à examiner : l'état initial, le frottement axial limite qui dépend de la dilatance potentielle du sol, les effets du temps, etc.
- La construction de diagrammes de stabilité cyclique est un résultat prometteur pour la conception de pieux soumis à charges cycliques.



- ▶ Aux différents étudiants, doctorants, post-doctorants etc.
- ▶ Aux différents laboratoires impliqués dans le Projet ANR
- ▶ Aux différents membres du groupe de rédaction



Merci de votre attention



SOLCYP – Journée de restitution
Lundi 13 mars 2017, Paris